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FATIGUE TESTS WITH HYPERBARIC DRY BUTT WELDED SPECIMENS

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1. Introduction

In the investigation described in this report the repair of a surface crack at the toe of a butt weld unter hyperbaric conditions was simulated and the fatigue properties of the repaired weld have been studied. This study is part of the work in a research project "Damage investigation and prevention in underwater structures". The project is supported by Deutsche Forschungsgemeinschaft (DFG).

2. Fabrication of test specimens

The specimens were fabricated from a 20 mm thick rolled plate with the dimensions 8000 mm x 2000 mm. The material was Alkilled steel St 52-3 according to DIN 17 100. Chemical composition and mechanical properties are given in table 1. The material was selected because it has a less favourable weldability compared with more recently developed steel qualities, but is often found in existing offshore structures.

From the large plate smaller plates (500 mm x 240 mm) were cut by means of flame cutting. From these 500 mm x 480 mm large test plates were welded by means of the submerged arc process.

Fig. 1 shows the edge preparation and table 2 the welding parameters. After completion of the top side welding the root was chipped and rewelded. The average angular distortion of the completed weld was about 1° with only small scatter.

In the next step the groove for the repair weld (fig. 2) was prepared by milling. According to the situation often found in practice in the repair of a surface crack a groove depth of about half plate thickness was selected according to discussions

with experts of Germanischer Lloyd.

The repair weld was carried out under dry hyperbaric conditions in the pressure chamber of the Laboratorium für Werkstoffkunde und Schweißtechnik der Universität der Bundeswehr Hamburg (Prof. pressure of bar. The MIG-Pulsarc Hoffmeister) under a 16 process was used for welding. The welding parameters were selected with regard to avoidance of weld defects and optimisation of the weld shape. Welding parameters and sequence of layers are given in table 3 and fig. 3.

3. Preparation for fatigue testing

In the Institut für Schiffbau der Universität Hamburg 50 mm wide test specimens were separated from the welded test plates by saw cut.

The specimens were measured in the device shown in fig. 4. From the distances d1 to d4 between the surface of the specimen and a reference line angular distortion and misalignment were calculated using the equations given in fig. 4 according to /1/(table 4). The secondary bending stresses were then calculated from equations given in /2/ and in fig. 4.

The maximum misalignment was found to be 0.9 mm with a much lower average. The misalignment can hence be considered as small. On other hand, angular distortions between 1° and 4.5° the were Maximum value as well as scatter are high. No explanation found. can be given especially for the high scatter. The situation with respect to angular distortion was at first considered as a offered the opportunity disadvantage, but of additional statements during the evaluation of test results.

Strain gauges of the type Hottinger LY6-120 were glued to one specimen from each test plate. The arrangement of strain gauges is shown in fig. 5. For the experimental investigation of residual stresses by means of the drilling hole method strain gauges TEA-06-062RK-120 were arranged at one specimen.

4. Fatigue testing

The fatigue tests were performed at a resonance machine Schenck PHX 60. In order to eliminate the influence of angular distortion of the specimens on the stress ratio, the specimens were clamped with the saw-cut edge upside.

The specimens with strain gauges were statically loaded prior to the fatigue test. The maximum stress under static load did not exceed that under the subsequent fatigue load. Linearity between load and strain and stability of strain gauge signal at zero load were checked by multiple stepwise loading.

The fatigue tests were carried out in air with a test frequency of about 30 Hz during resonance operation and about 2 Hz during hydraulic operation. The testing machine was operated hydraulically during the high load steps of the block program test only.

The test program included constant amplitude load tests with stress ranges of R=0 and R=-1 at two load levels each, a stair case test for 2 million cycles under zero - to tension load and 8-step block program tests according to Gassner under alternating load (R=-1). The exponential load distribution (/3/p.28) corresponds to the typical long-term distribution for offshore structures in the North Sea. Load data are given in table 10.

The number of specimens in the constant amplitude tests was at least 8 per load level. This is sufficient to justify a statistical analysis of the test results. Specimens from different test plates were combined for each test series resulting in a scatter of geometrical imperfections within each load level.

Drilling of the holes for the residual stress measurements was carried out by means of a device of Messrs. Measurements Group GmbH.

5. Test evaluation and results

5.1 Secondary bending stresses

The data from the strain gauge measurements are given in table 4. In order to compare calculated and measured bending stresses, the former had to be converted to those at the mid-point of the strain gauges. In the present case of a negligible influence of misalignment (see table 4) the point of zero bending moment lies at 1/2 and the correction factor for the bending stresses calculated according to table 4 becomes

1/2 1 CF = ----- = ----- = 0,72 1/2+b 1+2b/1

with l = 127 mm and b = 25 mm.

A comparison of calculated and measured stresses (table 5 and fig. 6) shows good agreement. Only in two cases a deviation of 5% is slightly exceeded. Because of the smaller efforts and costs involved it is recommended to calculate the geometrical imperfections and secondary bending stresses in small-scale specimens loaded in tension according to the procedure given in table 4.

5.2 Residual stresses

In order to obtain the order of magnitude of residual stresses, drilling hole measurements were carried out at speciment no. 10.7.

The strain gauge arrangement is shown in fig. 7, details of evaluation and results in table 6.

At the repair weld side tensile residual stresses between 33% and 61% of the yield stress of the base material have been found. At the lower side a lower value of 23% of the yield stress was

measured. With regard to the welding sequence these values appear reasonable. Parallel to the weld only small residual compression stresses were found.

5.3. Constant amplitude load fatigue tests

In most cases the crack initiated at the transition between repair weld and base material. Only these cases were used in the statistical evaluation. In some cases the crack initiated at the toe of the original weld. The data from these tests have been considered separately.

The test data are given in table 7, the results of the evaluation in table 8. The statistical evaluation for the individual stress levels was carried out under the usual assumption of a Gaussian distribution of fatigue lifes. An example is shown in fig. 8.

Figs. 9 and 10 show S-N curves for stress ranges R=0 and R=-1. The data are given in table 8. The stress ranges for 2 million cycles were obtained by extrapolation. The scatter band at each stress level corresponds to the ratio of fatigue lifes for 90% and 10% probability of survival.

A slope coefficient of about 3 has been found for both stress ranges investigated. This value, which is also used in codes as /4/ and /5/ is rather low for butt welds.

significant nominal stress ranges for 2 million cycles The are compared with the scatter bands from the systematic low reanalysis of test data /6/ as shown in fig. 11. This is at least partly due to the high secondary bending stresses caused by angular distortion. On the other hand, in spite of a considerable scatter of bending stresses, the scatter in the number of cycles to fracture for each individual test series ranges from 1 : 1,7 2,6 and is thus within usual limits. The effect of to 1 : the stress ration is smaller than that found in /6/ (fig. 11). This corresponds with the residual stress state observed.

For a better comparison of the results with the scatter band in /6/ the data from the present investigation were converted to a common level of secondary bending stresses. According to /7/ the level of secondary bending stresses in existing test data is up to about 30% of the membrane stress. Assuming a slope of 3 for each individual specimen the fatigue lifes have been corrected to a common stress range of 1,30 x $\sigma_{\rm m}$ by means of the following equation.

$$\begin{split} & N_{\rm corr} = N \left[(1 + \sigma_{\rm b} / \sigma_{\rm m}) / 1.30 \right]^3 \\ & \sigma_{\rm b} = {\rm secondary\ bending\ stress} \\ & \sigma_{\rm m} = {\rm membrane\ stress} \\ & N = {\rm number\ of\ cycles\ to\ fracture\ in\ the\ test} \\ & N_{\rm corr} = {\rm corrected\ number\ of\ cycles\ to\ fracture} \end{split}$$

The N_{corr} data are given in table 7 and the data for the corresponding S-N curves in table 8.

As fig. 11 shows, the stress ranges are still below the scatter band in /6/. Only if the stress range is based on membrane plus full bending stress, the stress ranges from the present investigation are close to the 50% (R=0) and 90% (R=-1) values according to /6/.

According to /4/ and /5/ the repair weld investigated would be classified as a class 80 detail based on membrane stress only. This classification is obtained from the stress range for 2 million cycles, 50% probability of survival and R=0 minus two standard deviations (1 standard deviation = factor 1,12 /8/). However, based on a common ratio of secondary bending stress and membrane stress of 1,30, the repair butt weld can be classified as a class 90 detail.

As table 8 shows, the slopes for the S-N lines from the data corrected for 30% secondary bending are slightly higher than for the lines based on membrane stress only. As could be expected, the scatter is reduced in all cases. This is also evident from figs. 12 and 13, which show scatter bands inluding all test data.

In fig. 12 also the data for specimens fractured at the toe of the initial weld are plotted. They fit very well into the scatter band of data from specimens, which failed from the repair weld toe.

5.4 Staircase test

The staircase test is a method for the assessment of the stress range for a given number of cycles (in the present case 2 million cycles) within given confidence limits. The first specimen is tested at an estimated stress range. If the number of cycles prescribed is reached without fracture, the following specimen is tested at an increased load level. If fracture occurs, the load for the next test is reduced. A fixed ratio between level adjacent load levels has to be chosen (in the present case 1,06). The test was carried out for a stress ratio R=0 and the membrane stress was used as the nominal stress. Again specimens from different test plates were selected for the test. The sequence of specimens tested is shown in fig. 14.

The test data have been evaluated by an improved method described in /9/. In this method not only the fractured or non-fractured specimens but all data plus one additional information are used. Details of the evaluation are given in table 9.

stress range of 117 N/mm² is obtained from the test, A mean which is about 22% higher than the stress range of 96 N/mm^2 from the extrapolation of the sloped part of the S-N curve. For 90% probability of exceedance a stress range of 73 N/mm² has been calculated. The unusually high ratio between mean and p=90% value of 1,6 can be attributed to the scatter of angular distortion between the specimens tested. The highest stress level during the whole test (specimen no. 10,2) was reached, because the preceding specimen (no. 6,5) had a small angular distortion and hence small secondary bending stresses. On the contrary the lowest stress level (specimen no. 4,5) was reached by testing of a specimen with large angular distortion (no. 10.8) at the second lowest stress level. If the four specimens

mentioned are omitted in the evaluation, a reasonable ration of between mean and P = 90%value of stress ranges is 1.21 obtained, while the mean value remains practically unchanged. The influence of scatter in the angular distortion on the results of the staircase test is obviously more pronounced than on the results of tests with constant amplitude load.

In order to get additional information, the non-fractured specimens were further tested after having reached 2 million cycles. During the continued test the stress range remained unchanged or was increased to 160 N/mm2. The results are plotted in fig. 15 together with the data from specimens fractured during the staircase test.

The data from the fractured specimens fit, with three exceptions, into the scatter band from the constant amplitude load test. A similar situation has been found for the specimens tested at an increased stress range. Only one of these specimens failed at a number of cycles below the scatter band. It can be concluded that the two other specimens were not considerably damaged during the staircase test. In all three cases where the number of cycles remained below the scatter band the angular distortion was higher than the average (see fig. 15).

The specimens tested at an unchanged stress range after 2 million cycles fractured at numbers of cycles up to about 4 million cycles. In cases, where run-outs are indicated in fig. 15, the specimens fractured at the toe of the submerged are weld or at the clamps. Mean line from constant amplitude load test and results from the staircase test indicate a knee in the S-N curve closely to 1 million cycles.

5.5 Block program test

Load sequence and test results are given in tables 10 and 11. Four data from specimens fractured at the toe of the repair weld are available. Cracks have also been observed at the toe of the submerged arc weld in all cases. A mean life of 3,766 million

cycles was found with a ratio of 1,5 between highest and lowest number of cycles. A damage calculation was carried out. Details are shown in table 12. In this calculation Miner's rule was used with a slope of 3,11 below and 5,22 above 2 million cycles. The damage sum calculated is close to unity.

6. Calculations according to the notch stress concept

For eight specimens from different test plates calculations according to the notch stress concept /10/ have been carried out. Sections transverse to the weld have been cut from the specimens. The sections were amplified optically with a magnification factor of 10 and the relevant notch data taken (table 13). An example of a weld contour is shown in fig. 16.

For the calculation the contour was simplified as also shown in fig. 16. The modification includes the increase of notch root radius by 1mm proposed in /10/ to take account of the elastic stress field around the notch.

The calculation model is shown in fig. 17. No angular distortion is considered in the model. The stress analysis was carried out by means of the boundary element program BETSY.

An example for the stress distribution is shown in fig. 17. The results of all calculations are given in table 13. A mean stress range of 144 N/mm2for 2 million cycles and p = 90% probability survival is obtained. This figure can be compared with the of stress range of 150 N/mm2 from fig. 11 for zero-to-tension load with secondary bending stresses excluded. This figure is related to p = 50%. Converted to p = 90% (Factor 1/1.121.3 ; 1.12=factor for standard deviation /8/) $\Delta \sigma$ =129 N/mm2 is obtained. This is below the mean calculated stress 11% range. However, in the light of the result of the staircase test the extrapolation of the sloped part of the S-N curve seems to underestimate the stress range for 2 million cycles.

A mean value of $K_F^{=1,80}$ is calculated from the data in table 13. This is slightly lower than K_F^{-} values obtained for butt welds in other investigations /11/. It should, however, be kept in mind that the present calculations were carried out with the really existing root radii plus 1 mm and not, as proposed in /10/ with a geometrical root radius of zero.

It can, however, be concluded from the calculation that the shape of the repair welds investigated is not inferior to that of usual butt welds.

7. Summary

Butt welds with hyperbaric dry MIG-repair welds have been tested under axial fatigue load.

The secondary bending stresses due to geometrical imperfection can be derived from simple measurements.

Residual tensile stresses of about 30-60% of the yield stress were measured perpendicular to the weld.

The slope of S-N curves for stress rations R=O and R=-1 is about 3, the knee of the curve is located closely to 1 million cycles. The stress range for 2 million cycles is comparatively low. As experiments as well as calculations according to the notch stress method show, this is mainly due to the comparatively high angular distortion and not to an inferior weld shape. With angular distortions corresponding to a ratio between secondary bending stress and membrane stress of 1,30 the repair weld can be classified as a class 90 detail according to the IIW Fatigue Design Recommendation /4/.

Results from block program tests fit well with a damage calculation using Miner's rule.

Most specimens fractured from the toe of the repair weld. However, fracture or at least crack initiation was also observed

at the toe of the initial submerged arc weld. From this it can be concluded that the fatigue properties of original and repair weld are nearly equivalent.

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Table 1 Base Material

Chemical Composition in %

C	<u>si</u>	Mn	P	S	<u>Al</u>	Nb	<u>v</u>
0.18	0.38	1.47	0.016	0.006	0.036	0.020	0.004

Mechanical Properties

REH	RM	ISO-V-Notch Toughness					
N/mm ²	N/mm ²	Joule at 20°C					
405	580	146	154	136	mean	145	

Table_2	Submerged	Arc	Welding	Parameters

La	yer	Current A	Voltage V	Wire Speed mm/min
1	Root Layer	500	27	700
2	Intermediate Layer	600	30	500
3	Counter Layer	600	34	500
4	Top Layer	600	. 34	500

Wire:	S 2	Mo	4	mm
Powder:	LW	320		
Powder Height	22	mm		

Table 3 Repair Welding Parameters

Pressure		16	ba	r		
Puls Volt	age	54	V			
Puls Curr	ent	280	A			
Puls Cycl	e .	4,	7 :	ms		
Base Volt	age	15	V			
Base Curr	ent	50	A			
Base Cycl	е	5,	0	ms		
Wire Diam	eter	1,	0	mm		
Wire Spee	đ	9,	8	m/min		
Weld Spee	đ	Lay	er	1	387	mm/min
		11	t i	2	285	mm/min
		**		3	285	mm/min
		11	l	4	387	mm/min

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Gas	Flow	Ar	112,5	Nl/min
		ArS3	7,5	Nl/min

Table 4.1Geometric Imperfections

Plate	e No. 2	1= 129	əmm 1	b= 25 mm				
Spec	d ₁	d ₂	d ₃	d ₄	е	φ	σ _{b2}	^σ b3
No.	[mm]	[mm]	[mm]	[mm]	[mm]	[゜]	σ_{m}	σ_{m}
2.1 2 3 4 5 6 7 8	20,07 20,25 20,20 20,20 20,30 20,25 20,16 20,23	15,67 15,73 15,63 15,64 15,58 15,68 15,65 15,63	15,07 15,28 15,18 15,20 15,30 15,38 15,25 15,18	20,95 21,00 20,91 20,89 20,85 20,85 20,84 20,87	0,83 0,64 0,63 0,62 0,41 0,44 0,57 0,62	-3,68 -3,67 -3,69 -3,67 -3,68 -3,60 -3,62 -3,68	0,44 0,47 0,48 0,48 0,52 0,50 0,48 0,49	0,80 0,76 0,76 0,76 0,72 0,72 0,72 0,74 0,75
Plate	e No. 3	1= 127	7 mm d	o= 25 mm				
3.1 2 3 4 5 6 7 8	27,10 27,00 26,90 26,95 27,00 26,70 27,03 27,00	20,85 20,85 20,90 20,90 21,00 21,13 20,80 20,80	20,90 20,90 20,85 20,73 20,80 20,70 20,85 20,80	26,95 26,95 26,85 26,80 26,70 26,55 26,80 26,87	-0,08 -0,07 +0,05 +0,17 +0,18 +0,47 -0,09 -0,02	-4,40 -4,37 -4,30 -4,34 -4,26 -4,09 -4,36 -4,39	0,76 0,74 0,71 0,72 0,72 0,65 0,76 0,75	0,70 0,71 0,71 0,72 0,69 0,71 0,69 0,71

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Plate	e No.4	l= 126	5 mm l	o= 25 mm			1	
Spec No.	d ₁ [mm]	d ₂ [mm]	d ₃ [mm]	. ^d 4 [mm]	e [mm]	φ [°]	σ _{b2} σ _m	$\frac{\sigma_{b3}}{\sigma_{m}}$
4.1 2 3 4 5 6 7 8	25,85 26,00 25,95 25,87 25,90 25,82 25,72 25,98	21,52 21,58 21,63 21,49 21,40 21,35 21,30 21,28	21,12 21,10 21,28 21,20 21,15 21,16 21,18 21,00	25,47 26,17 26,15 26,10 26,12 26,10 26,19 26,10	0,40 0,58 0,44 0,37 0,32 0,26 0,21 0,34	-3,11 -3,40 -3,29 -3,32 -3,39 -3,37 -3,38 -3,51	0,51 0,48 0,49 0,50 0,50 0,49 0,53	0,52 0,64 0,61 0,61 0,61 0,61 0,63 0,63
Plate	e No.5	1= 127	7 mm]	b= 25 mm				
5.1 2 3 4 5 6	25,12 25,25 25,20 25,30 25,40 25,45	22,60 22,50 22,50 22,49 22,55 22,48	22,10 22,10 22,00 22,00 22,05 21,95	26,13 26,10 26,15 26,32 26,40 26,35	0,74 0,59 0,73 0,73 0,73 0,75	-2,35 -2,42 -2,45 -2,55 -2,58 -2,64	0,21 0,25 0,24 0,24 0,25 0,27	0,57 0,55 0,58 0,60 0,60 0,60
Plate	e No.6	1= 126	5,5 mm]	b= 25 mm				
6.1 2 3 4 5 6 7 8 9	25,75 25,65 25,40 25,20 25,15 25,15 25,10 24,94 25,00	22,60 22,65 22,90 23,05 23,20 23,20 23,24 23,25 23,25	22,22 22,45 22,52 22,58 22,61 22,65 22,65 22,55 22,55 22,46	25,40 25,27 25,30 25,20 25,20 25,15 25,35 25,48 25,52	0,38 0,17 0,42 0,54 0,69 0,64 0,72 1,00 0,99	-2,27 -2,08 -1,89 -1,71 -1,63 -1,59 -1,63 -1,62 -1,72	0,37 0,28 0,23 0,19 0,20 0,17 0,11 0,13	0,38 0,32 0,35 0,34 0,35 0,33 0,37 0,43 0,44
Plate	e No.7	l= 126	5,5 mm]	b= 25 mm				
7.1 2 3 4 5 6 7 8	19,85 19,80 19,70 19,75 19,70 19,73 19,82 19,82	16,53 16,48 16,58 16,55 16,50 16,50 16,20 16,30	16,40 16,40 16,48 16,42 16,35 16,50 16,45 16,30	19,75 19.75 19,70 19,82 19,78 19,79 19,87 19,93	0,13 0,08 0,12 0,16 0,19 0,01 -0,28 0,02	-2,39 -2,39 -2,27 -2,36 -2,37 -2,33 -2,52 -2,56	0,39 0,39 0,36 0,37 0,37 0,38 0,44 0,41	0,40 0,40 0,39 0,42 0,42 0,42 0,39 0,39 0,44

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Plate	No.8	1= 126	5 mm ł	o= 25 mm			,		
Spec	d ₁	d ₂	d ₃	d ₄	е	φ	σ _{b2}	σ _{b3}	
No.	[mm]	[mm]	[mm]	`[mm]	[mm]	[°]	σ _m	σ _m	
8.1 2 3 4 5 6 7 8	26,35 26,18 26,15 26,12 26,10 26,12 26,12 26,08	21,87 21,67 21,80 22,00 22,10 22,00 22,08 22,00	21,35 21,20 21,58 21,70 21,75 21,67 21,80 21,70	26,12 25,85 25,55 25,95 26,05 25,90 26,10 25,82	0,57 0,49 0,16 0,32 0,40 0,35 0,32 0,31	-3,31 -3,28 -2,98 -3,00 -2,97 -2,99 -2,99 -2,99	0,51 0,52 0,54 0,48 0,45 0,48 0,46 0,48	0,58 0,56 0,44 0,51 0,53 0,51 0,52 0,49	
Plate	No. 9	1= 126	5,5 mm k	o= 20 mm					
9.1 2 4 5 7 8 9	19,44 19,23 19,19 19,25 19,35 19,27 19,37 19,24 19,40	17,06 16,90 16,47 16,80 17,00 17,02 17,03 16,90 17,00	16,71 16,63 16,43 16,63 16,53 16,53 16,65 16,89 16,76 16,79	20,00 20,50 20,00 19,80 19,78 19,50 19,73 19,75 19,95	0,46 0,15 0,26 0,58 0,45 0,20 0,22 0,31	-2,03 -2,22 -2,25 -2,01 -2,00 -1,83 -1,85 -1,91 -1,99	0,23 0,19 0,27 0,25 0,23 0,23 0,25 0,24 0,24	0,45 0,55 0,48 0,42 0,44 0,38 0,37 0,40 0,42	
Plate	No. 10	l= 126	5,5 mm k	o= 25 mm					
10.1 2 3 4 5 6 7 8	26,35 26,40 26,36 26,25 26,25 26,60 26,35 26,53	21,65 21,63 21,67 21,70 21,60 21,35 21,53 21,40	21,23 21,25 21,23 21,35 21,23 20,85 21,13 21,05	26,65 26,70 26,74 26,65 26,88 27,10 26,95 26,95	0,53 0,49 0,57 0,46 0,53 0,66 0,55 0,47	-3,62 -3,66 -3,65 -3,55 -3,69 -4,12 -3,81 -3,95	0,51 0,53 0,51 0,51 0,49 0,56 0,51 0,56	0,68 0,68 0,70 0,67 0,73 0,80 0,75 0,74	
Plate	No. 11	l= 126	5,5 mm k	o= 25 mm					
11.1 2 3 4 5 6	18,65 18,60 18,61 18,59 18,40 18,50	17,60 17,40 17,68 17,48 17,68 17,70	17,39 17,00 17,02 17,02 17,00 16,95	19,20 19,08 19,08 19,00 19,00 18,90	0,33 0,54 0,84 0,60 0,88 0,93	-1,02 -1,17 -1,07 -1,11 -0,97 -0,98	0,08 0,09 0,04 0,08 0,01 0,03	0,26 0,30 0,31 0,29 0,31 0,30	

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Plate	e No. 14	1= 12	28 mm	b= 25 mm	•				
Spec	ď1	d ₂	d ₃	d ₄	e	φ	σ _{b2}	σ _{b3}	
No.	[mm]	[mm]	[mm]	`[mm]	[mm]	[°]	$\sigma_{\rm m}$	σ_{m}	
14.1 2 3 4 5 6 7 8	23,15 23,22 23,20 23,25 23,15 23,27 23,50 23,55 23,55	21,26 21,60 21,30 21,40 21,30 21,25 21,30 21,00 21,00	21,06 21,20 21,05 21,10 21,08 21,15 21,00 21,25 21,15	23,60 23,60 23,50 23,62 23,60 23,55 23,60 23,60 23,60	0,30 0,52 0,34 0,40 0,32 0,16 0,36 -0,28	-1,58 -1,44 -1,56 -1,56 -1,56 -1,58 -1,72 -1,75 -1,75	0,19 0,15 0,20 0,18 0,18 0,22 0,24 0,32	0,34 0,33 0,33 0,34 0,34 0,31 0,34 0,27 0,30	

Dista No. 14 l = 100

<u>Table 5</u>

Measured vs. calculated bending stress

Spec No.	mean stress ^o m [N/mm ²]	measured strains $\epsilon_1 + \epsilon_2 + \epsilon_3 + \epsilon_4$ $\cdot 10^{-6}$			relati ^Ø b2 meas.	ve bend / o _m cacl.	ting str ⁰ b3 meas.	cesses / o _m cacl.	
		650						0.54	
2.6	232	658	1515	1627	527	0,39	0,36	0,51	0,52
3.4	100	210	750	778	215	0,56	0,52	0,57	0,52
4.5	100	268	639	689	237	0,41	0,36	0,49	0,44
5.6	100	336	578	665	271	0,26	0,19	0,42	0,43
6.9	100	416	521	631	327	0,11	0,09	0,32	0,32
7.2	232	745	1438	1508	739	0,32	0,28	0,34	0,29
8.4	100	303	538	694	282	0,28	0,35	0,42	0,37
9.6	232	859	1349	1467	789	0,22	0,17	0,30	0,27
10.8	100	262	694	755	207	0,45	0,40	0,57	0,53
11.2	232	1000	1176	1380	860	0,08	0,06	0,23	0,22
14.9	100	342	588	607	362	0,26	0,21	0,25	0,22

strain gauge arrangement see fig.5

D = 5,13mm D₀ = 1,7 mm a = 0,14 b = 0,34 v = 0,3 A = $-\frac{1+v}{2E}$ a = $-4,33\cdot10^{-7}$ B = $-\frac{1}{2E}$ b = $-8,10\cdot10^{-7}$ tan $2\overline{B} = -\frac{\varepsilon_1 - 2\varepsilon_2 + \varepsilon_3}{\varepsilon_3 - \varepsilon_1}$ B = \overline{B} for $\varepsilon_1 \ge \varepsilon_3$ B = $90^\circ - \overline{B}$ for $\varepsilon_1 \le \varepsilon_3$

$$\sigma_{X} = \frac{\varepsilon_{1} + \varepsilon_{3}}{4A - 4B} \sqrt{(\varepsilon_{1} - \varepsilon_{2})^{2} + (\varepsilon_{2} - \varepsilon_{3})^{2}}$$

Gauge	Strain · 10 ⁻⁶			Stress[N/mm ²]		B°
No.	ε1	ε2	³ کا	$\sigma_{\mathbf{x}}$	σу	
1 2 3 4	- 178 - 154 - 272 - 131	+ 3 - 146 + 90 - 26	+ 96 + 110 + 117 + 84	+ 136 + 137 + 248 + 94	- 42 - 86 - 69 - 39	- 9 + 22 - 20 + 1



Table 7.1

Spec.	$\Delta \sigma_{\rm b} / \Delta \sigma_{\rm m}$	$\Delta \sigma_{\rm b}^{+} \Delta \sigma_{\rm m}$	N	Ncorr	crack			
No.		[N/mm ²]	cycles	cycles	side			
8.1	0,58	367	74400	133572	3*)			
2.1	0,80	418	103200	273947				
9.7	0,37	318	120000	140447				
7.2	0,40	325	131000	163616				
9.6	0,38	320	154300	184575]		
11.1	0,26	292	162900	148321		1		
11.2	0,30	302	174500	174500		}		
7.1	0,40	325	179900	224691	3	ł		
2.6	0,50	348	118700	182345	2*)	}		
2.7	0,48	343	127700	188428	2			

Series 01 R= 0
$$\Delta \sigma_{m} = 232 \text{ N/mm}^2$$

$$N_{corr} = N \cdot \left(\frac{1 + \sigma_b / \sigma_m}{1, 3}\right)^3$$

*2 crack at initial weld, 3 crack at repair weld

,

Series02R=
$$\Delta \sigma_{m} = 160 \text{ N/mm}^{2}$$
4.10,5224330050048033634.80,63261308800608709|8.20,56250337100582509|4.20,64262361400725587|11.60,30208438600438600|9.20,55248488900828677|9.10,45232559100775824|7.30,3922264000078233832.80,4923839380059293127.40,372195225006115302

Table 7.2

Series	11 R = -1	$\Delta \sigma_{\rm m} = 160$	D N/mm			
Spec.	$\Delta \sigma_{\rm b} / \Delta \sigma_{\rm m}$	$\Delta \sigma_{\mathbf{b}}^{+} \Delta \sigma_{\mathbf{m}}^{-}$	N	Ncorr	crack	1
No.		[N/mm ²]	' cycles	cycles	side	
4.7 2.5 8.5 4.6 9.3 8,4 5.1 8.3	0,63 0,72 0,53 0,61 0,48 0,51 0,57 0,44	261 275 245 258 237 242 251 230	508400 512400 644900 712700 762400 766400 811400 861700	1002163 1186764 1051324 1353799 1124962 1201037 1429236 1171153	3	
Series	12 R= -1	$\Delta \sigma_{\rm m} = 24$	40 N/mm ²			
3.8 9.9 2.4 14.1 4.4 14.6 14.5	0,71 0,42 0,76 0,34 0,61 0,31 0,34	410 341 422 322 386 314 322	$108700 \\ 145900 \\ 155400 \\ 194200 \\ 212000 \\ 246000 \\ 246800 \\ 2$	247393 202455 385619 212683 402702 270139 270290	3	
6.8	0,43	346	i 306400	410434	1 3	i

2 .

<u>Table 8</u>

Results of constant ampllitude load tests

a) Evaluation based on membrane stress $\boldsymbol{\sigma}_m$ only

	Series					
	01	02	11	12		
N ₁₀ cycles N50 " N90 " N ₁₀ /N ₉₀	198950 132541 88299 1:2,3	610102 414285 281317 1:2,2	896010 685793 524897 1:1,7	313590 194232 120303 1:2,6		
K	К 3,07		3,11			

b) Evaluation based on membrane + 30% bendingstress $\sigma_{m} + \sigma_{b}$ $\begin{smallmatrix} N_{10} & cycles \\ N_{50} & " \\ N_{90} & " \\ N_{10} / N_{90} & | \\ \end{smallmatrix}$ 244844 879027 1392238 426873 175562 637472 1182714 290191 125884 462296 1004721 197274 1:1,9 1:1,9 1:1,4 1:2,2 K3,47NNumber of cycles for 10 (50) (90) %probability of survivalTFlore of S-N-curve

Ta	b	Le	- 9

Evaluation of staircase test stress ratio R= 0

11

Spec. No.	$\Delta \sigma_{\rm b} / \sigma_{\rm m}$	$\Delta \sigma [N/mm^2]$	$\Delta \sigma_2[N/mm^2]^1)$	N _f
			2)	
11.5	0,31	120,0		1389800
7.6	0,39	$113,4 \text{ NF}^{1}$)	160,0	201300
9.8	0,40	120,0		1087800
11.4	0,29	113,4 NF		4068200
5.5	0,60	120,0		1199500
7.7	0,39	113,4 NF		4189600
6.2	0,32	120,0 NF	160,0	253200
6.3	0,35	127,0		1189600 ₃
6.4	0,34	120,0 NF		3126800 R)
6.5	0,35	127,0 NF		-
10.2	0,68	134,0		451900
10.1	0,68	127,0		599100
10.5	0,73	120,0		612200
10.3	0,70	113,4		1026800
9.5	0,44	107,2 NF	160,0	555900
10.4	0,67	113,4 NF		
3.2	0,71	120,0		1005600
3.3	0,71	113,4		792700
8.6	0,51	107,2 NF		2353000 R
6.9	0,44	113,4 NF		3217800
3.4	0,72	120,0		471600
8.4	0,51	113,4	1	1366200
10.8	0,74	107,2		856900
4.5	0,61	101,2 NF	1	2880200
5.6	,60	107,2 NF		2788300
14.9	0,30	103,4 NF	í	2863000 R
14.8	0,27	120,0 NF		2443800 R
14.2	0,33	127,0		1081400
14.3	0,33	120,0 NF		2331400
- !	-	127.0 -	- !	- !

No fracture at 2 million cycles
 Stress range after 2 million cycles
 R= runout

Evaluation after /11/:

step i	number ^f i	i.f	2 i·f _i	stress range $\Delta \sigma [N/mm^2]$
0	1	0	0	101,2
1	4 .	4	4	107,2
2	9	18	36	113,4
3	10	30	90	120,0
4	5	20	80	127,0
5	1	5_	25	134,0
	F=30	A=77	B=235	•

Table 9.2

d= 1,0583 log d = 0,02461Step factor Mean stress range $\Delta \overline{\sigma} = \Delta \sigma_{\ddot{o}} d^{A/F} = 101, 2 \cdot 1,0583^{77/30} = 117,0 \text{ N/mm}^2$ $K = \frac{F \cdot B - A^2}{F^2} = 1,2456$ Variance Standard deviation $/11/fig.15 \log s = 0,02461 \cdot 2,3 = 0,05660$ s = 1,139 $K_1 = 1,28$ Confidence range 90% Standard error mean stress range $\log s_m = C_m \cdot \log s$ log s 0,05660 $C_{m} = 0,23 \ (/11/ \text{ fig.16}) \text{ for } \frac{1}{100} = \frac{1}{0,02461} = 2,30$ $log s_{m} = 0,23.0,05660 = 0,01302$ $s_{m} = 1,030$ Standard error standard deviation $\log s_s = C_s \cdot \log d$ $C_{s} = 2,80$ (/11/ fig.17) log s_s = C_s · log d = 2,80 · 0,02461 = 0,06891 $S_{s} = 1,172$ $K_{2} = 1,28$ for $p_{ij} = 90\%$ $\log \Delta \sigma_{90} = \log \overline{\Delta \sigma} - K_2 \log s - K_1 / (\log s_m)^2 + (K_2 \log s_s)^2$ $= 2,0682 - 1,28 \cdot 0,05660 - 1,28 \sqrt{0,01302^2 + (1,28 \cdot 0,06891)^2}$ = 1,8816 $\Delta \sigma_{90} = 76.1 \text{ N/mm}^2$ $\Delta \sigma_{90} / \Delta \overline{\sigma} = 0.65$ Evaluation after omission of specimens 6.5/10.2 and 10.8/4.5: $\Delta \bar{\sigma} = 117,2 \text{ N/mm}^2$ K = 0,7825s = 1,073 $s_m = 1,020$ $s_s = 1,061$ $\Delta \sigma_{90} = 96,6 \text{ N/mm}^2$ log s = 0,03076 $\begin{array}{rcl} \log \ {\bf s}_{\rm m} &=& 0,00846\\ \log \ {\bf s}_{\rm S} &=& 0,02584\\ \log \ \Delta \ \sigma_{90} &=& 1,9851 \end{array}$

$$\Delta \sigma_{90} / \Delta \overline{\sigma} = 0,82$$

Table 10 Block Program Test

Stress ratio R=-1 Load sequence

Step No.	No. of cycles	Stress range [N/mm ²]
4 3 2 1 2 3 4 5 6 7 8 7	87 15 3 1 3 15 87 487 2 730 15 400 462 000 15 400	387,5 465,0 542,5 620,0 542,5 465,0 387,5 310,0 232,5 155,0 77,5 155,0
6 5	2 730 487 499 445	232,5 310,0

Table :	11
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Results from block programm test

Spec.	N	N	N	
No.	resonance	hydr.	total	
7.8	4 477 000	59 788	4 536 788	root crack
4.3	3 458 700	49 456	3 508 156	
14.4	3 956 500	56 187	4 012 687	
2.2	2 963 500	42 907	3 006 407	
(12.1	1 976 300	29 758	2 006 058)	
r		mean	3 766 000	cycles

failure	criterion	8 D · Σ i=3	$\frac{n_{i}}{-\frac{1}{N_{i}}} = 1$,	(1)			
$D = nur n_i = nur N_i = nur i$	mber of blo mber of cyc mber of cyc	ocks to fra cles in sta cles to fra	acture ep: per bl acture in	Lock (see below) step: under com amplitude) nstant e load				
Δα	$\sigma_i = \Delta \sigma_{max}$	(1,125 -	i -) 8		(2)				
Δα	o _i = stress	s range for	r step i						
$N_{i} = 2 \cdot 10^{6} \left(-\frac{\Delta \sigma_{i}}{\Delta \sigma_{D}} \right)^{-K_{i}} $ (3)									
$\sigma_{\rm D}$ = stress range for 2 million cycles under constant amplitude load									
From (1) and (3)									
$1 \qquad 2 \cdot 10^6$									
	n _i	- -	$\Delta \sigma_{i}$, ^K i					
	i N _i	- 2 11 i	ί ^Δ σ _D)					
	$\Delta \sigma_{\rm D} = 114$	N/mm ²	$\Delta \sigma_{max} =$	= 620 N/mm ²					
				∆σ _i κ _i					
step i	"i	$\Delta \sigma_{i}$	^K i	$n_i \left(\frac{\Delta \sigma_D}{\Delta \sigma_D} \right)^{-1}$					
		N/mm ²			 - 				
1 2	1 6	620 543	3,11	194 770					
3	30	465		2 376					
4 5	174 974	388 310		7 849 21 863					
6	5 460	233		50 431					
8	462 000	78	5,22	63 728					
	499 445			227 288					
$D = \frac{2 \cdot 10^6}{227 \ 288} = 8,80$									
$N_{calc} = 499 \ 445 \ \cdot \ 8,80 = 4 \ 395 \ 000$									
$N_{calc}/N_{exp} = 4 395 000 / 3 766 000 = 1,17$									

Spec. No.	root radius [mm]	flank angle [°]	к _F	$\begin{bmatrix} \Delta \sigma_{\rm D} \\ [N/mm^2] \end{bmatrix}$
1.2 2.3 3.5 5. 6.4 7.7 10.6	2,5 2,5 1,9 1,8 4,4 5,4 1,5	32 42 42 30 24 26 47	1,71 1,73 2,01 1,97 1,56 1,52 2,25	149 147 128 130 163 166 115
14.8	4,3	28 	1,64 ean 1.80	155 mean 144

1)
$$\Delta \sigma_{\rm D} = \Delta \sigma \; (N=2.10^6; p=90\%, R=0)$$

270

$$\Delta \sigma_{\rm D} = \frac{1}{K_{\rm F}^{+} + 0,1}$$



Fig. 1 Edge preparation for SAW butt weld



Fig. 2 Edge preparation for repair weld



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Misalignment:
$$e = (d_2 - d_3)(1 + \frac{b}{a}) + (d_4 - d_1)\frac{b}{a}$$

Angular Distortion: $\varphi^{\circ} = \frac{180}{\pi \cdot a} (d_2 - d_1 + d_3 - d_4)$
Rel. Bending Stresses :
Point (2) $\frac{\sigma_{b2}}{\sigma_m} = -\frac{31}{2at} [3(d_2 - d_1) - (d_3 - d_4)]$
Point (3) $\frac{\sigma_{b3}}{\sigma_m} = +\frac{31}{2at} [(d_2 - d_1) - 3(d_3 - d_4)]$

Fig. 4 Determination of geometrical imperfections



Fig. 5 Arrangement of strain gauges for secondary bending stress measurements



σ_m = membrane stress σ_b = secondary bending stress

Fig. 6 Comparison of measured and calculated secondary bending stresses





Fig. 7 Arrangement of strain gauges for residual stress measurements









S-N curve for zero-to-tension load



Fig. 10 S

S-N curve for alternating load







Fig. 12 Comparison of scatter bands for data from zero-to-tension load test

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Fig. 13 Comparison of scatter bands for data from alternating load test





Staircase test





3P

Specimen 1.2





b) simplified weld contour for calculation 10:1



